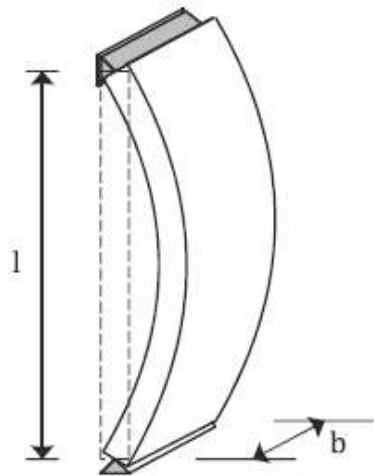
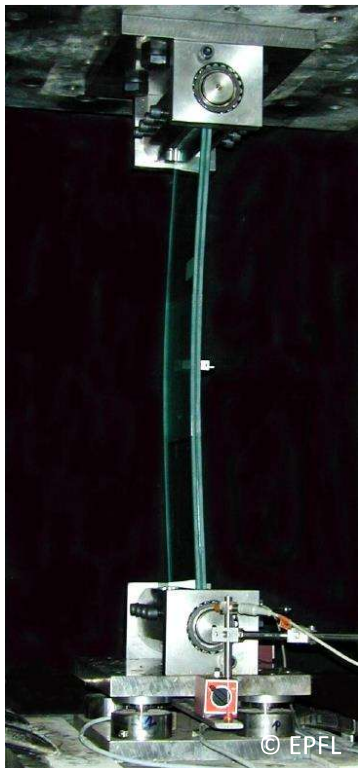


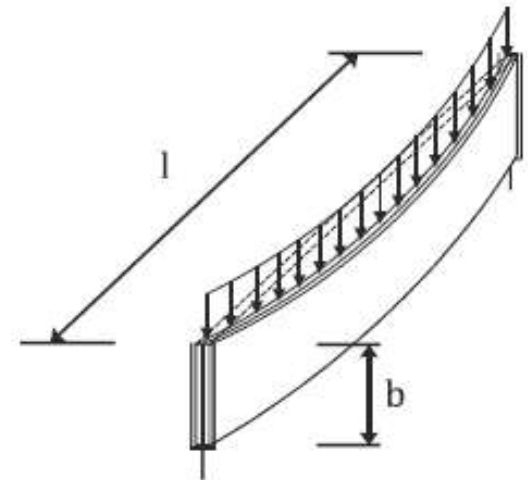
CEN/TS 19100-3

in-plane loaded elements and joints

in-plane loaded elements



column buckling



lateral torsional buckling

in-plane loaded elements

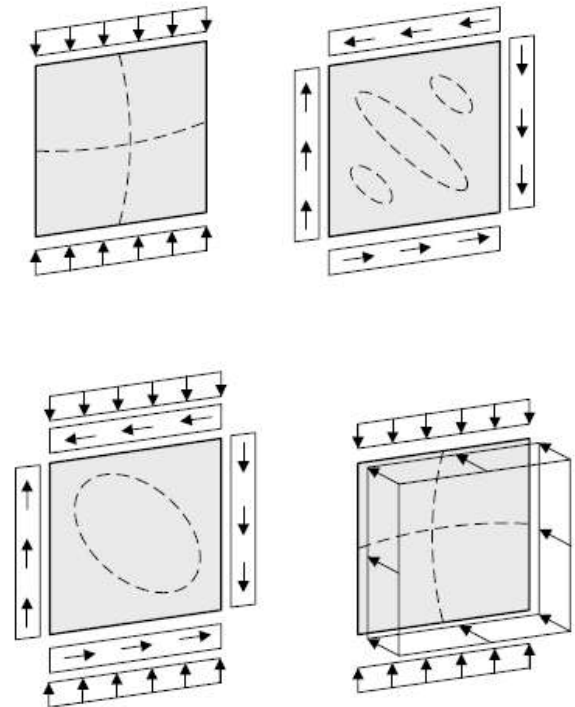
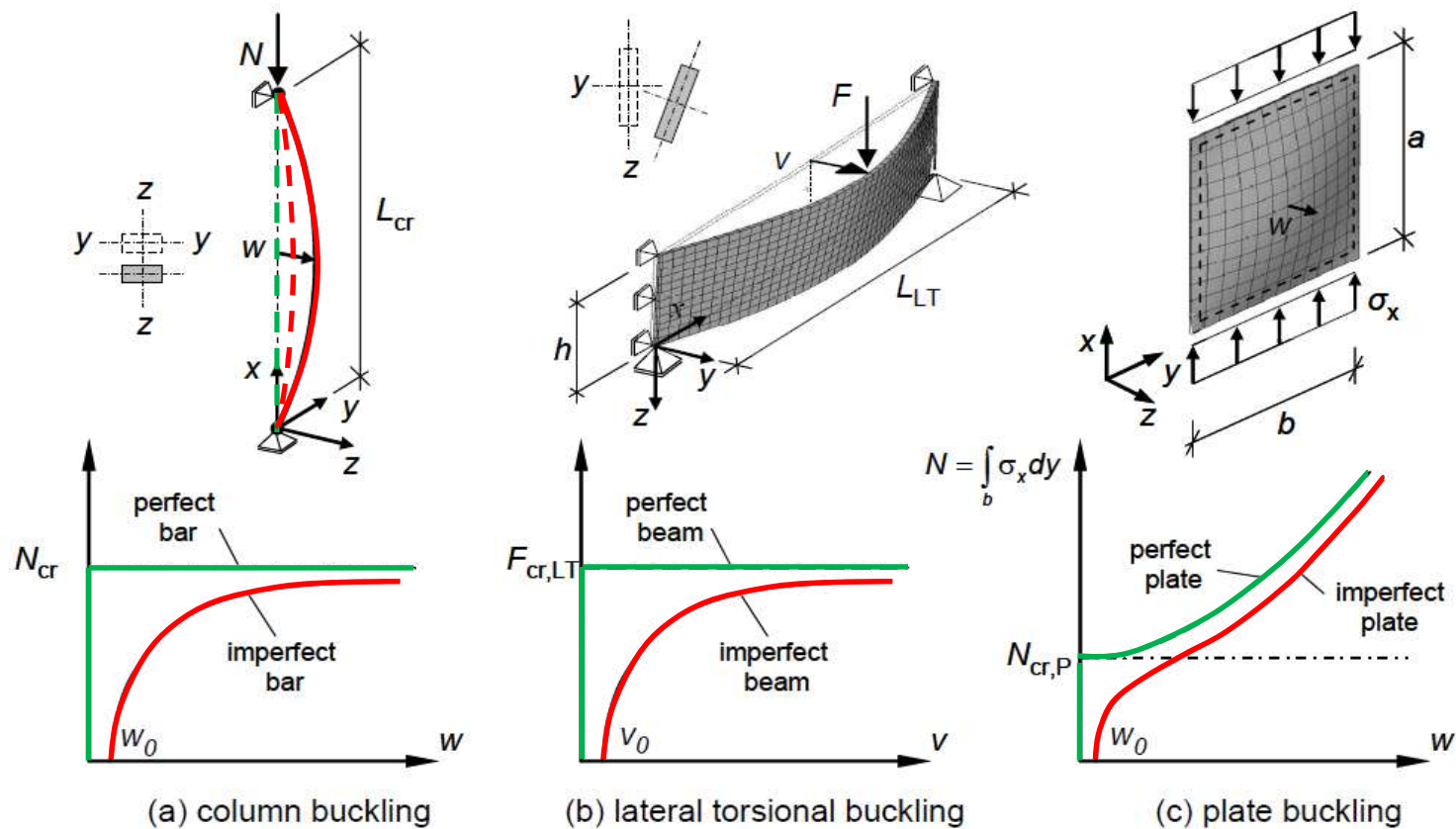


plate buckling

in-plane loaded elements



in-plane loaded elements

- ✓ structural analysis
 - second order effects

$$\alpha_{cr} = \frac{F_{cr}}{F_{Ed}} \leq 10$$

- imperfections
 - equivalent geometrical imperfection
 - basic imperfection (ULS and SLS)*

$$e_0 = \sqrt{e_{0,length}^2 + e_{0,installation}^2}$$

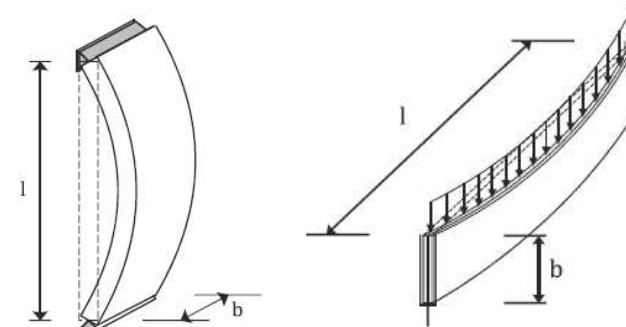


Table 7.1 (NDP) — Imperfection parts for buckling cases

Type	l_0	both for mono and laminated glass panes	
		$e_{0,length}^a$	$e_{0,installation}^{b,c,d}$
Flexural buckling and plate buckling	Distance of inflexion points in the relevant critical mode in direction of the applied load	$l_0/333$	$h_e/2$
Lateral torsional buckling	Distance of inflexion points at the edge in compression in the relevant critical mode	$l_0/450$	$h_e/2$
Shear buckling	Longest diagonal	$l_0/1000$	$h_e/5$

^a $e_{0,length}$ should be applied at the location where the curvature of the relevant critical mode gets its maximum.

^b $e_{0,installation}$ may be applied at the location where the installation eccentricity occurs. Alternatively, for simplification reasons, it may be applied at the same location as the one of $e_{0,length}$.

^c For perpendicular to the glass plane, straight edges over the thickness of the laminate, the value for h_e is: $h_e = h_{tot}$. For stepped edges or other edge geometries the value of h_e can be determined individually.

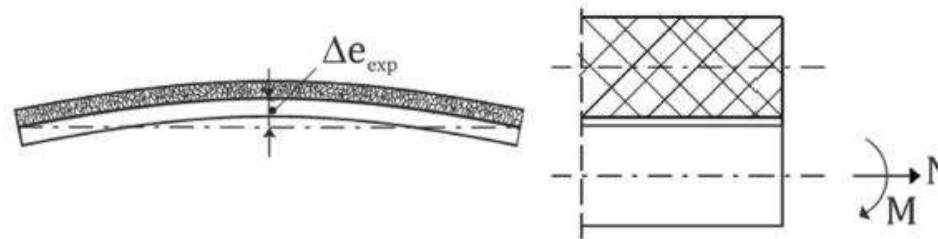
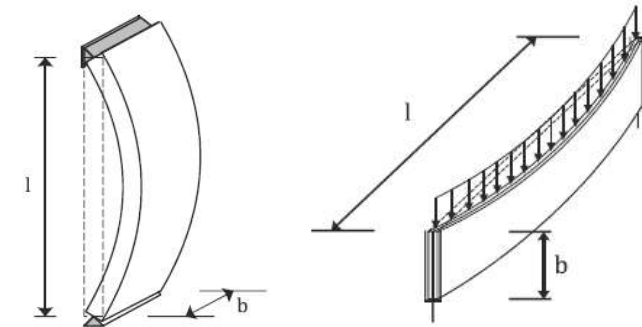
^d If $e_{0,installation}$ is recorded on site it may be reduced to the measured value, but not smaller than 3mm. This requires care in execution and control.

in-plane loaded elements

- ✓ structural analysis
 - second order effects

$$\alpha_{cr} = \frac{F_{cr}}{F_{Ed}} \leq 10$$

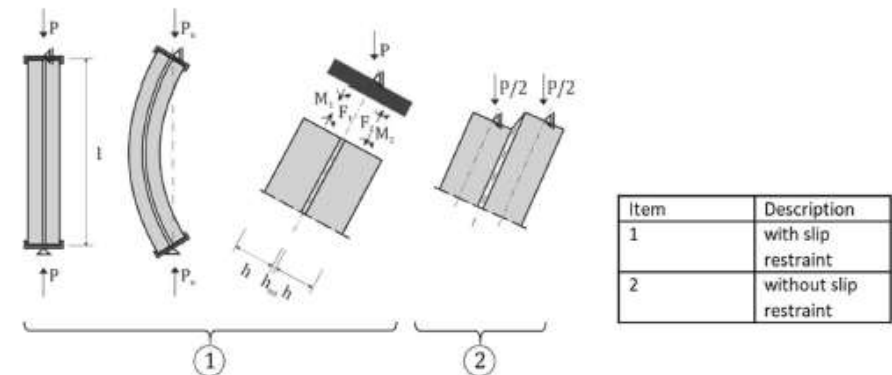
- imperfections
 - equivalent geometrical imperfection
 - basic imperfection (ULS and SLS)*
 - shift after fracture laminated glass and/or TTG ply



in-plane loaded elements

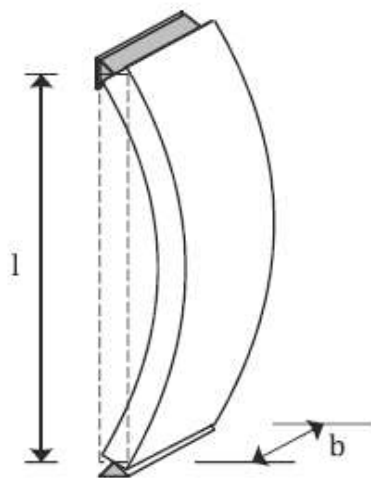
✓ points of attention

- interlayer properties (level 0, 1, 2)
- temperature and long-term effects
- detailing
 - load introduction
 - continuous vs concentrated
 - avoid peak stresses
 - durable detailing
 - avoid hard contact
 - deformations
 - tolerances (manufacturing and erecting)
 - slip restraint



in-plane loaded elements

✓ critical buckling load N_{cr}



$$N_{cr} = \frac{\pi^2 E I_{z,eff}}{L^2}$$

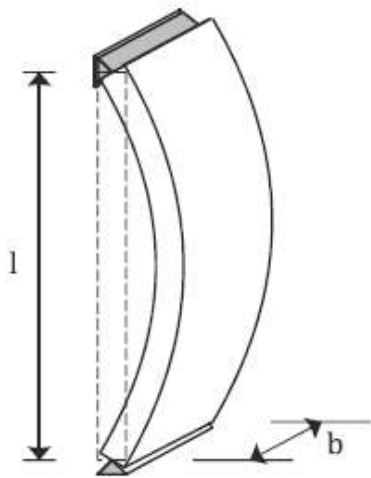
$$I_{z,eff} = \frac{\sum I_i}{1 - \frac{\Psi \cdot \beta \cdot K_s}{\left(\frac{\pi}{L}\right)^2} + \alpha^2}$$

Table B.1 – Cross section parameters for effective inertia calculation

K_s	$\frac{G_L}{t} B$	$\frac{G_L(t)}{t} B$
α^2	$\frac{K_s}{E} \left(\frac{(z_1 + z_2)^2}{I_1 + I_2} + \frac{1}{A_1} + \frac{1}{A_2} \right)$	$\frac{K_s}{E} \left(\frac{2(z_1 + z_2)^2}{2I_1 + I_2} + \frac{1}{A_1} \right)$
β	$\frac{(z_1 + z_2)}{E(I_1 + I_2)}$	$\frac{(z_1 + z_2)}{E(2I_1 + I_2)}$
Ψ	$(z_1 + z_2)$	$2 \cdot (z_1 + z_2) = 2 \cdot z$

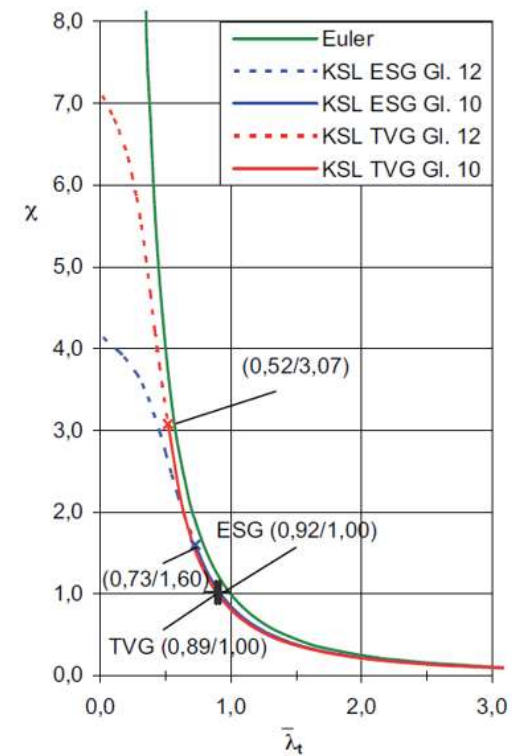
in-plane loaded elements

✓ critical buckling load N_{cr}



$$N_{cr} = \frac{\pi^2 E I_{z,eff}}{L^2}$$

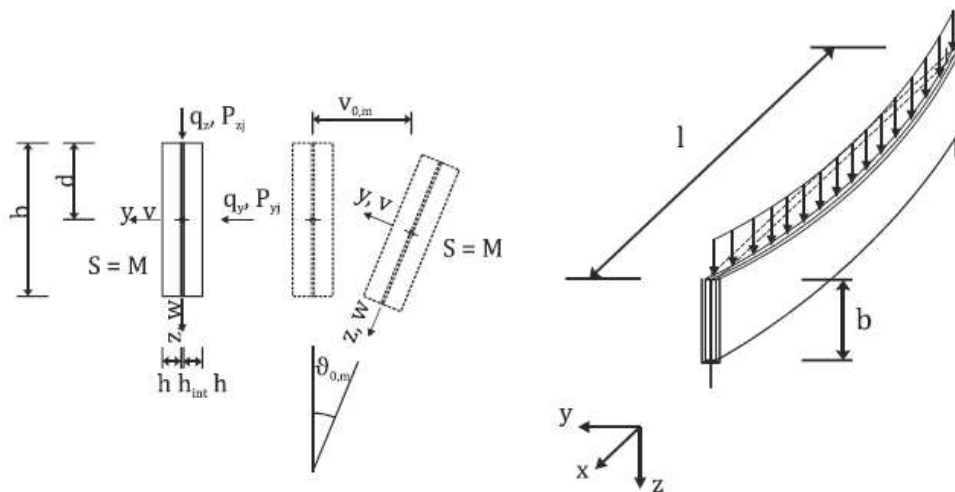
slenderness ratio λ
 reduction factor $\chi = f(\lambda)$
 buckling strength $N_{b,Rd}$



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in-plane loaded elements

✓ critical bending moment M_{cr}



$$M_{cr,LT} = C_1 \frac{\pi^2 E I_{z,eff}}{L^2} \left[\sqrt{(C_2 z_p)^2 + \left(\frac{G I_{t,eff} L^2}{\pi^2 E I_{z,eff}} \right)} + (C_2 z_p) \right]$$

Table A.1 — Values C_1 and C_2

Bending moment	C_1	C_2
Constant	1.0	–
Linear (zero at midspan)	2.7	0.0
Parabolic (zero at both extremities)	1.13	0.46
Triangular (zero at both extremities and maximum at mid span)	1.36	0.55

$$I_{t,eff} = \frac{2}{3} B h^3 + 2 B h (h_L + h)^2 \left(1 - \frac{\tanh\left(\lambda_T \frac{B}{2}\right)}{\lambda_T \frac{B}{2}} \right)$$

$$\lambda_T = \sqrt{\frac{2 G_L}{G h_L h}}$$