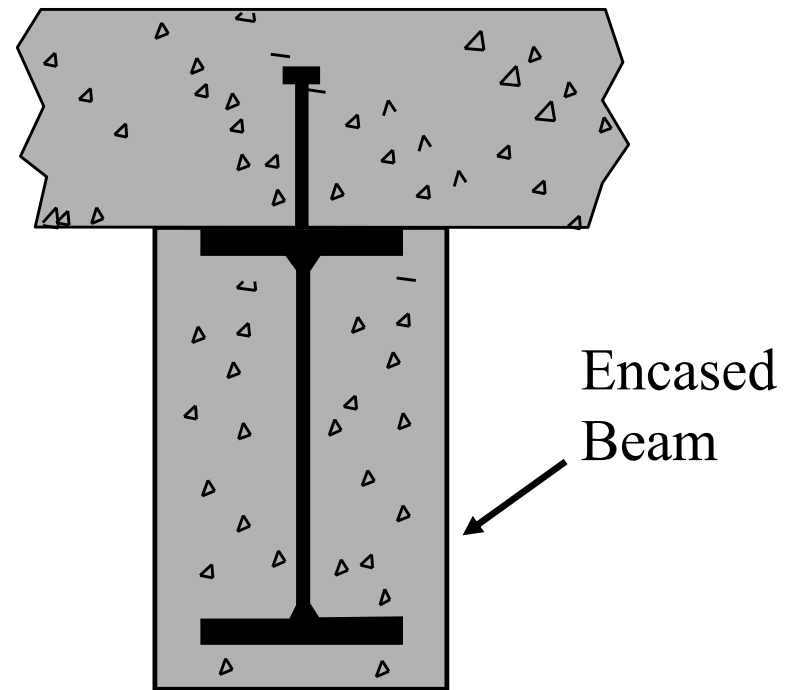
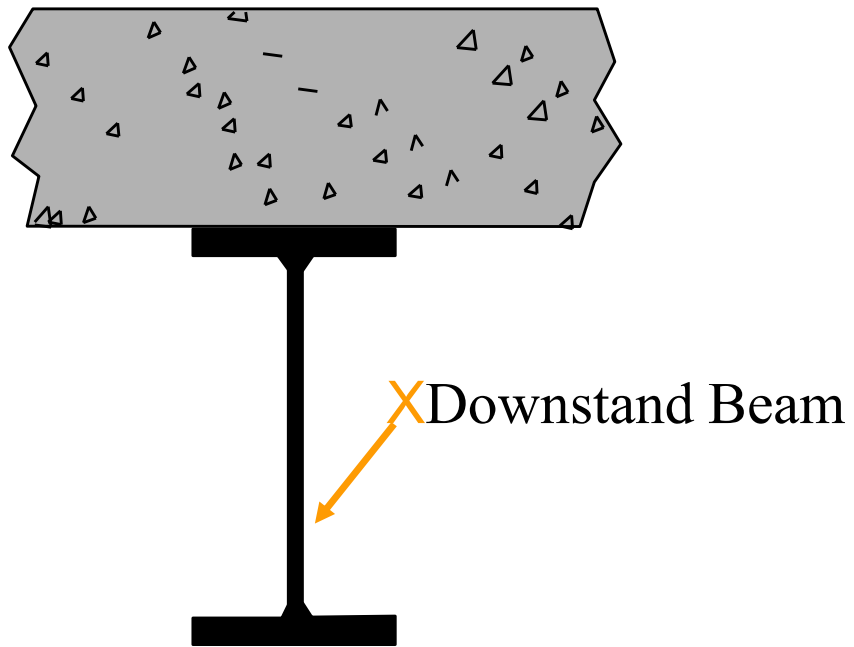
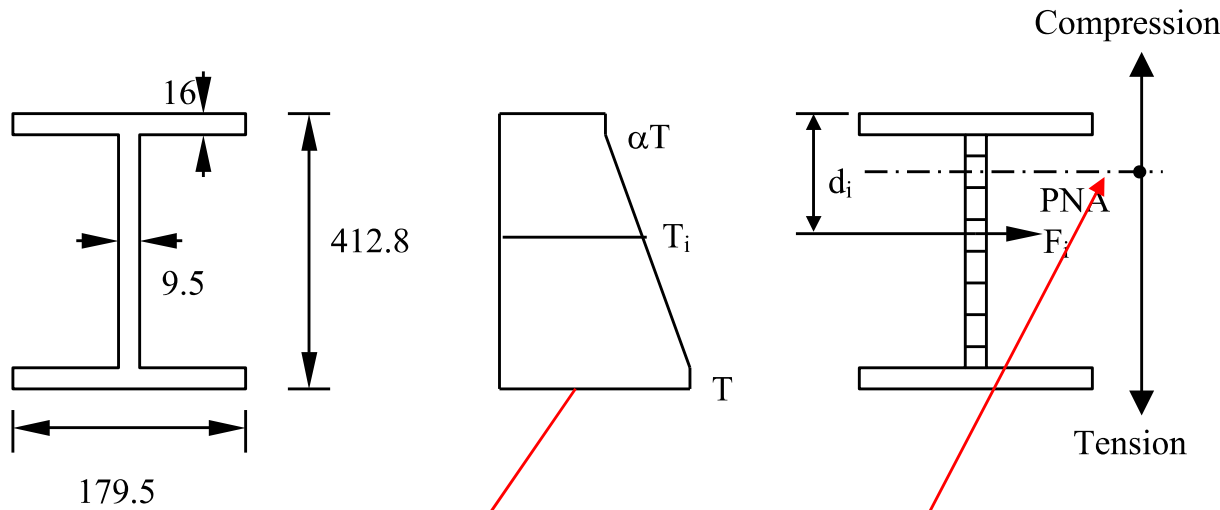


Steel and Composite Beams





In general, because the temperature distribution in the steel section is non-uniform, the steel section is divided

Part i	Temperature T_i ($^{\circ}\text{C}$)	Resultant force $F_i = p_y(T_i) * A_i$ (kN)	Lever arm top d_i (mm)	Bending moment $= F_i * d_i$ (kN.m)
Upper flange	$650 * 0.8 = 520$	-565.81	8	-4.53
Web 1	528.13	-86.08	39.8	-3.43
Web 2	544.38	<u>-24.10</u>	<u>70.76</u>	<u>-1.71</u>
		55.96	94.56	5.29
Web 3	560.63	73.93	135.0	9.98
Web 4	576.88	67.74	182.6	12.37
Web 5	593.13	61.56	230.2	14.17
Web 6	609.38	55.75	277.8	15.49
Web 7	625.63	50.21	325.4	16.34
Web 8	641.88	44.68	373.0	16.66
Lower flange	650.0	266.16	404.8	<u>107.74</u>
Total	NA	0	NA	188.39

from Bending moment into layers of approximately the same temperatures.

Simple Method in Eurocode 3 Part 1.2

The full plastic bending moment method involves lengthy calculations. For steel temperature distribution based on a steel section supporting a concrete slab, the approximate calculation method is:

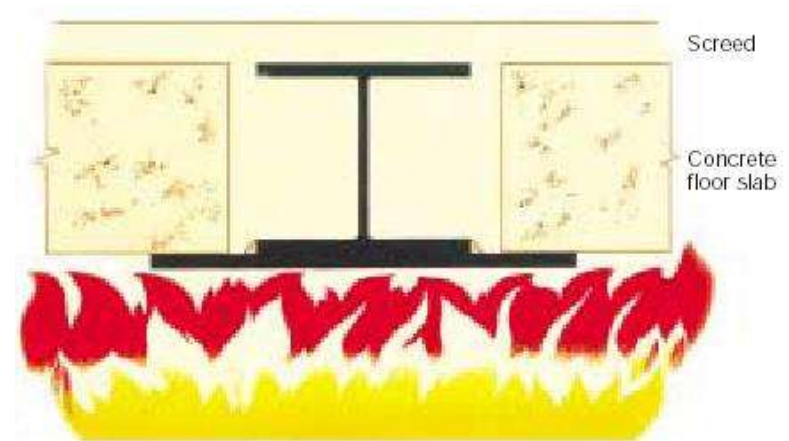
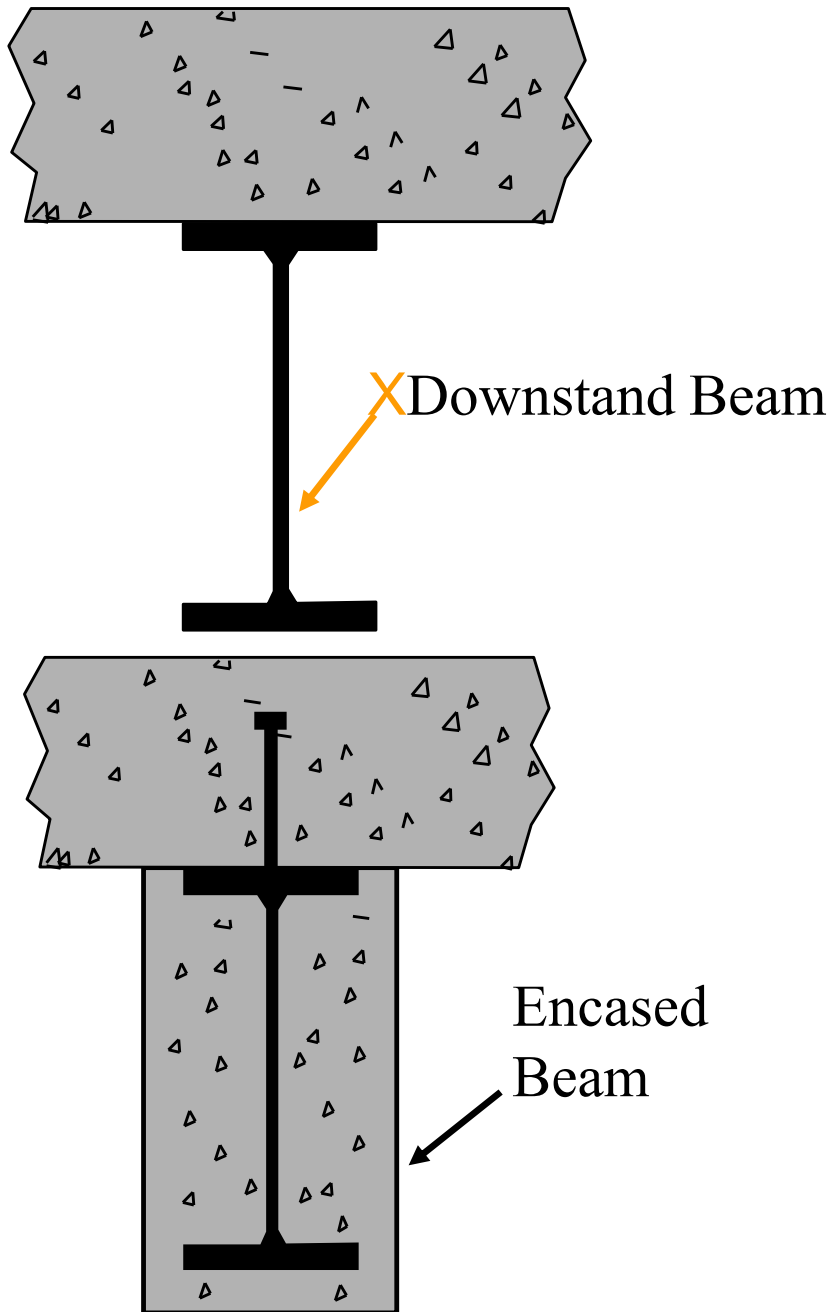
$$M_{P,fi} = k_{y,T} M_p / (\kappa_1 \kappa_2)$$

$k_1=1.0$ for uniform heating across section/for composite beam, $k_1=0.7$ for steel beam with three sided heating

$k_2=1.0$ for uniform heating along length, $k_2=0.85$ for hogging bending moment near supports

Previous example: Section UB406x178x74, steel grade S275, $W_{pl}=1501\text{cm}^3$, steel retention factor =0.337 at 650°C, $M_{p,fi}=1501 * 0.337 * 0.275 / 0.7 = 198.7 \text{ kN.m}$. Compare this to 188.39 kN.m from previous calculations.

Limit of simple method



Critical temperature

4.2.4 Critical temperature

- (1) As an alternative to 4.2.3, verification may be carried out in the temperature domain.
- (2) Except when considering deformation criteria or when stability phenomena have to be taken into account, the critical temperature $\theta_{a,cr}$ of carbon steel according to 1.1.2 (6) at time t for a uniform temperature distribution in a member may be determined for any degree of utilization μ_0 at time $t = 0$ using:

$$\theta_{a,cr} = 39,19 \ln \left[\frac{1}{0,9674 \mu_0^{3,833}} - 1 \right] + 482 \quad (4.22)$$

where μ_0 must not be taken less than 0,013.

Calculating Load Ratio: Beam Example

Ambient temperature design data:

9m, Dead load = 10.5 kN/m, Imposed Load = 15 kN/m,

UKB 457x152x52, $f_y=355\text{N/mm}^2$, $M_{pl}=389\text{ kN.m}$

Calculating load ratio (assuming $\varphi_{fi}=0.5$):

$$\text{UDL}=1.0*10.5+0.5*15=18\text{kN/m}$$

$$M_{\text{fire}}=1/8 * 18 * 9^2 = 182\text{ kN.m}$$

$$\text{Load ratio} = 182/389 = 0.47$$